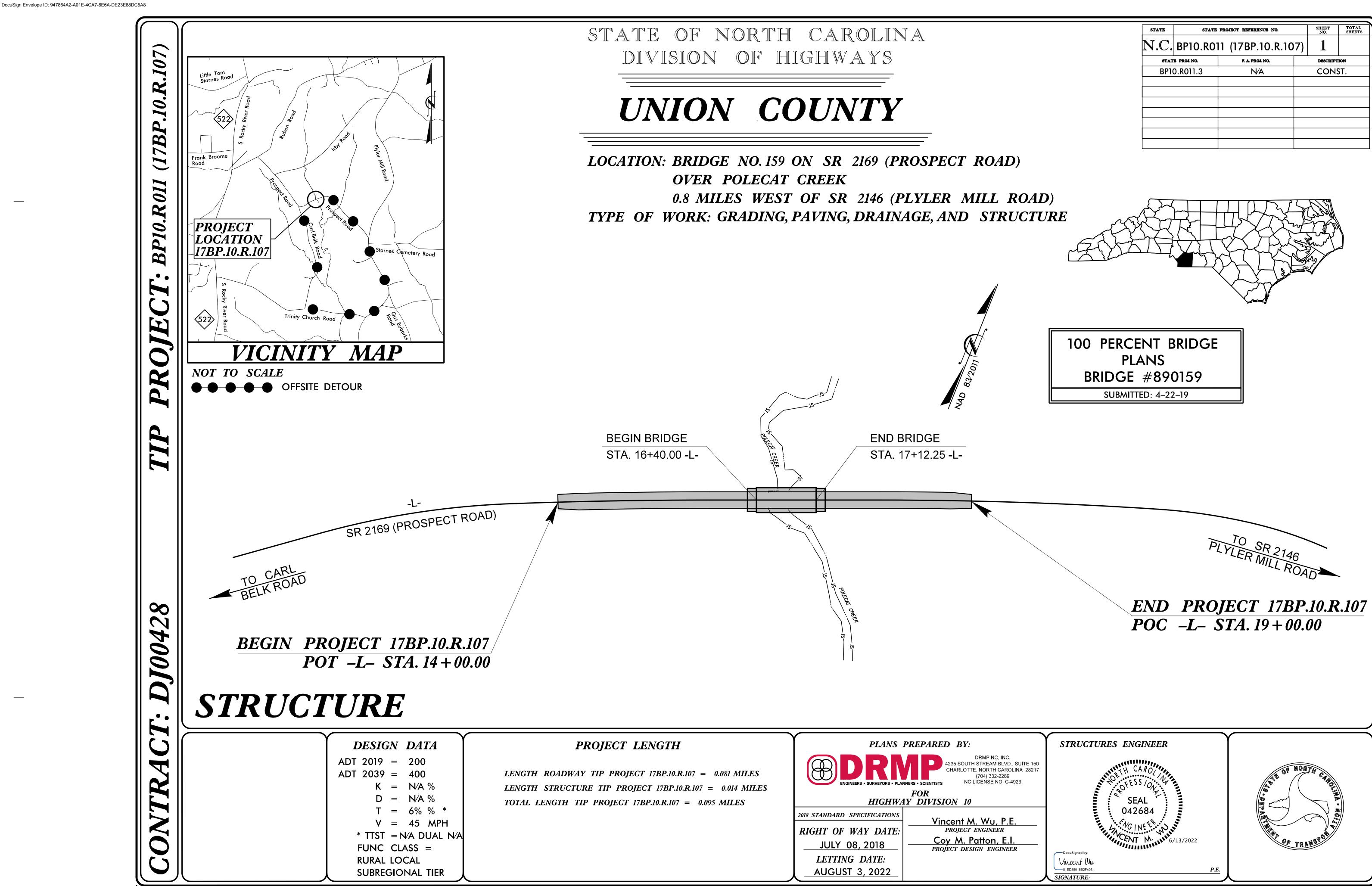
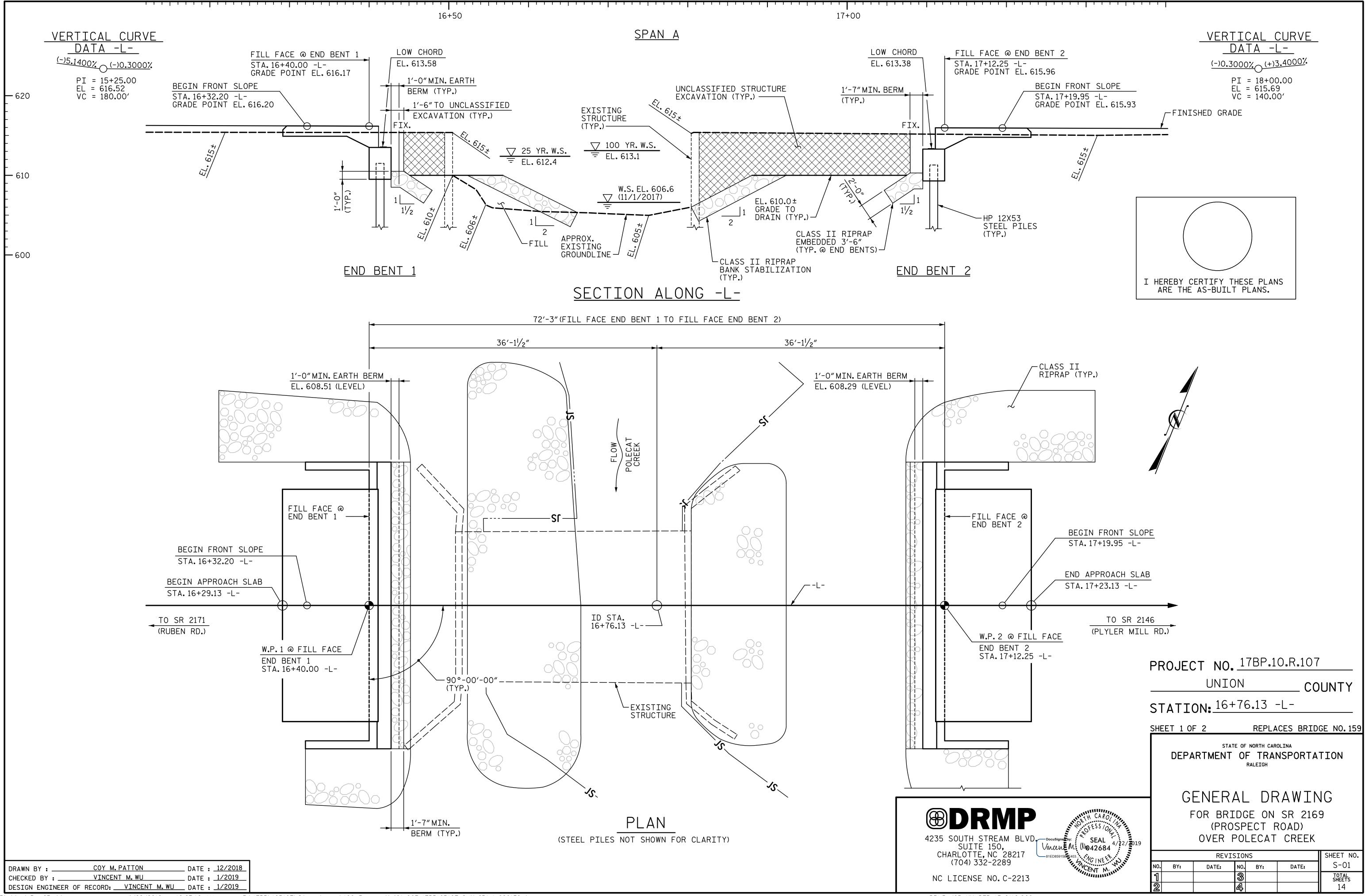
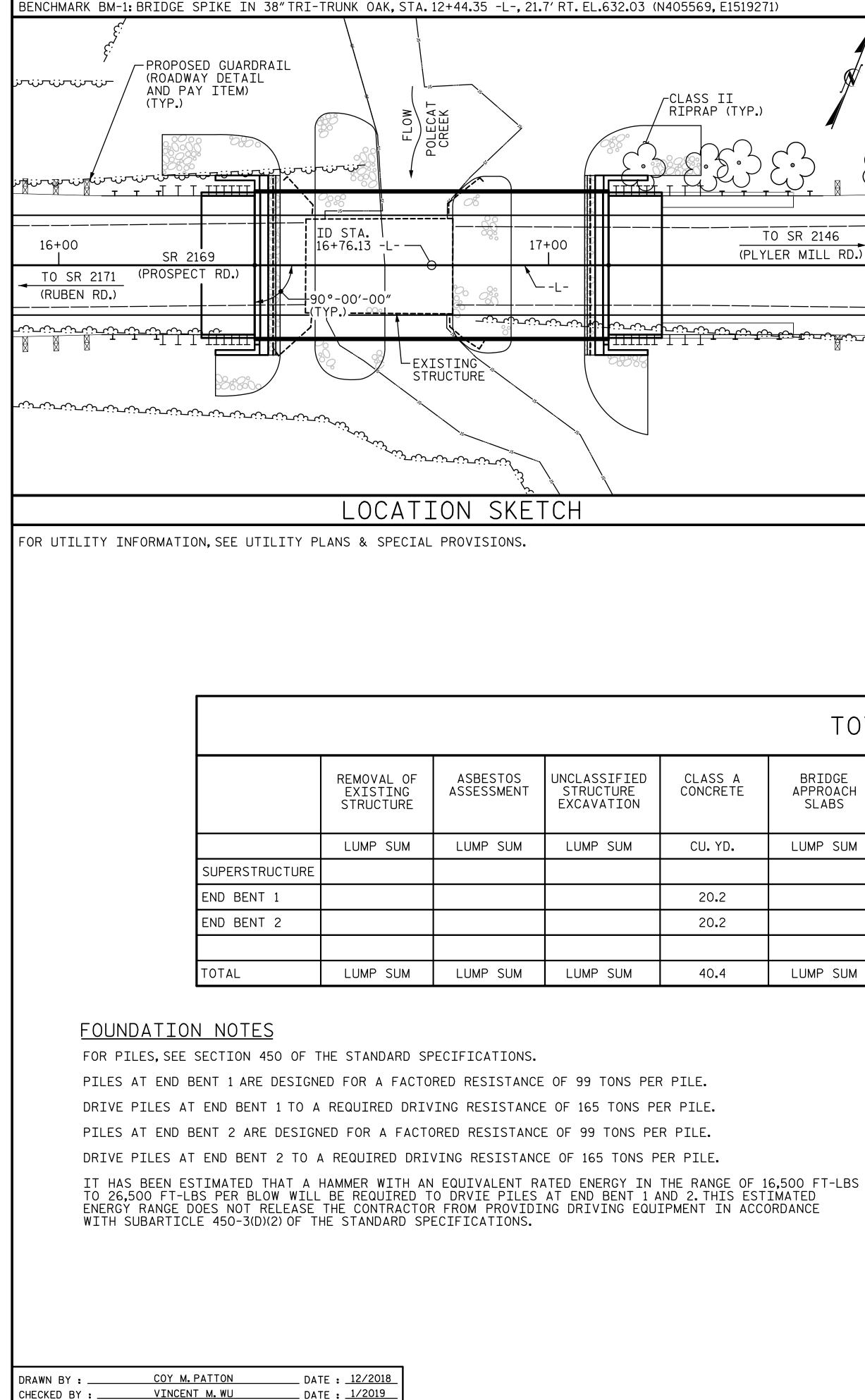
This electronic collection of documents is provided for the convenience of the user and is Not a Certified Document –

The documents contained herein were originally issued and sealed by the individuals whose names and license numbers appear on each page, on the dates appearing with their signature on that page. This file or an individual page shall not be considered a certified document.





	DESIGN	ENGINEER	O۲	RECORD:	VINCENT
Ρ	lotted B	y: CPattor	I		4/22/20



DESIGN ENGINEER OF RECORD: _____VINCENT M. WU DATE : 1/2019

71)	<u>GENERAL NOTES</u>
	ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.
đ	THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SF
y v	THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.
	FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE "STANDARD NOTES" SHEET.
\sim	FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.
	THE EXISTING STRUCTURE CONSISTING OF (1)31'-6"SPAN WITH TIMBER DECK ON STEEL I-BEA SUPPORTED BY RUBBLE MASONRY ABUTMENTS SHALL BE REMOVED.THE EXISTING BRIDGE IS PR
TO SR 2146	REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO F SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTIC SPECIFICATIONS.
YLER MILL RD.)	INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHAL BID FOR "REMOVAL OF EXISTING STRUCTURE".
ڹڴڹڹٮڗڰٮٮڹ <u>ڴڹڹؾؾۦڗؾۦؾ</u>	THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA (ON SHEET 1 OF 2)SHALL BE EXCAVATED F ROADWAY OF APPROXIMATELY 33± FT.(LEFT)AND 25± FT.(RIGHT)AT END BENT 1,23± FT.(LEFT TO THE ELEVATION OF 610′± AT BOTH END BENTS AS DIRECTED BY THE ENGINEER.SEE SECTIO
	THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT S
	THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18 - EVALUATING SCOUR AT
	AT THE CONTRACTOR'S OPTION, PRESTRESSED CONCRETE END BENT AND BENT CAPS MAY BE SUB CAST-IN-PLACE CAPS.THE CONTRACTOR SHALL COORDINATE WITH THE RESIDENT ENGINEER TO FROM THE STRUCTURES MANAGEMENT UNIT.THE REDESIGN AND ANY ADDITIONAL MATERIALS NE TO THE CONTRACTOR.
	FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
	FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
	FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
	FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
	ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.
	FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPEC

TOTAL BILL OF MATERIAL										
BRIDGE APPROACH SLABS	REINFORCING STEEL	PILE DRIVING EQUIPMENT SETUP FOR HP 12X53 STEEL PILES		P 12x53 EL PILES	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	PRE	O″X 2'-O″ ESTRESSED RETE CORED SLABS
LUMP SUM	LBS.	NO.	NO.	LIN.FT.	LIN.FT.	TONS	SQ. YDS.	LUMP SUM	NO.	LIN.FT.
					140				10	700.00
	2,449	5	5	75		132	147			
	2,449	5	5	75		114	127			
LUMP SUM	4,898	10	10	150	140	246	274	LUMP SUM	10	700.00

HYDRAULIC DATA

DESIGN DISCHARGE: FREQUENCY OF DESIGN FLOOD: DESIGN HIGH WATER ELEVATION: DRAINAGE AREA: BASE DISCHARGE (Q100): BASE HIGH WATER ELEVATION:

550 CFS 25 YRS. 612.4′ 1.03 SQ.MI. 805 CFS 613.1′

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE: FREQUENCY OF OVERTOPPING FLOOD: OVERTOPPING FLOOD ELEVATION:

2,450 CFS 500+ YRS. 615.9′ (SAG @ STA.17+41.4 -L-)

> 4235 SOUTH STREAM SUITE 150, CHARLOTTE, NC 2821 (704) 332-2289 NC LICENSE NO. C-22

PECIFICATIONS.

MS WITH A CLEAR ROADWAY OF 19.167' AND ESENTLY POSTED FOR LOAD LIMIT. FALL INTO THE WATER. THE CONTRACTOR CLE 402-2 OF THE STANDARD

CONTRACTOR'S ATTENTION IS DIRECTED TO WITH APPLICABLE STATE OR FEDERAL LL BE INCLUDED IN THE LUMP SUM PRICE

FOR A DISTANCE FROM THE CENTERLINE OF T) AND 21 ± FT. (RIGHT) AT END BENT 2. AND ON 412 OF THE STANDARD SPECIFICATIONS.

INFORMATION AVAILABLE. SINCE THIS HAVE NO CLAIM WHATSOEVER AGAINST THE IN DIFFERENCES BETWEEN THE EXISTING SITE.

BRIDGES".

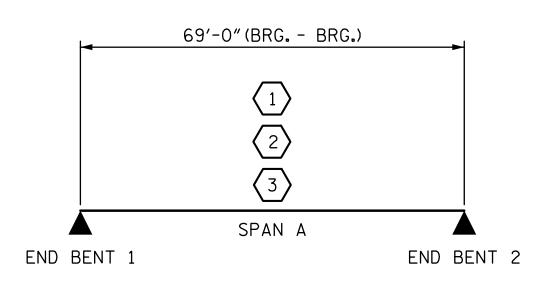
BSTITUTED IN PLACE OF THE RECEIVE REVISED PLANS AND DETAILS

EEDED WILL BE AT NO ADDITIONAL COST

CIAL PROVISIONS

	PROJE	CT NO.	17BP.1	0.R.107	
		UNIO			UNTY
	STATI	0N: <u>16+</u>	76.13	-L-	
	SHEET 2 C)F 2			
	DEPA	STATE ARTMENT	OF NORTH CAR OF TRAI RALEIGH		TION
	G	ENERA	AL DF	RAWIN	IG
BLVD. Docusigned by: SEAL 4/22/2019	F		DGE ON SPECT F OLECAT	ROAD)	9
17 B1ED8591559F403.	NO. BY:	REVIS	IONS NO. BY:	DATE:	SHEET NO. S-02
213	1		3 4		total sheets 14

																							
	LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS																						
	STRENGTH I LIMIT STATE SERVICE III LIMIT STATE								TE														
										MOMENT					SHEAR						MOMENT		
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)
		HL-93(Inv)	N/A	1	1.006		1.75	0.273	1.03	А	EL	34.5	0.507	1.32	А	EL	6.9	0.80	0.273	1.01	А	EL	34.5
DESIGN	-	HL-93(0pr)	N/A		1.341		1.35	0.273	1.34	А	EL	34.5	0.507	1.72	А	EL	6.9	N/A					
LOAD	-	HS-20(Inv)	36.000	2	1.306	47.02	1.75	0.273	1.34	А	EL	34.5	0.507	1.65	А	EL	6.9	0.80	0.273	1.31	А	EL	34.5
RATING	F	HS-20(0pr)	36.000		1.74	62.64	1.35	0.273	1.74	А	EL	34.5	0.507	2.14	А	EL	6.9	N/A					
		SNSH	13.500		2.917	39.379	1.4	0.273	3.75	А	EL	34.5	0.507	4.87	А	EL	6.9	0.80	0.273	2.92	А	EL	34.5
		SNGARBS2	20.000		2.187	43.741	1.4	0.273	2.81	А	EL	34.5	0.507	3.47	А	EL	6.9	0.80	0.273	2.19	А	EL	34.5
		SNAGRIS2	22.000		2.077	45.69	1.4	0.273	2.67	А	EL	34.5	0.507	3.23	А	EL	6.9	0.80	0.273	2.08	А	EL	34.5
		SNCOTTS3	27.250		1.452	39.565	1.4	0.273	1.87	А	EL	34.5	0.507	2.43	А	EL	6.9	0.80	0.273	1.45	А	EL	34.5
	S S	SNAGGRS4	34.925		1.218	42.554	1.4	0.273	1.57	А	EL	34.5	0.507	2.03	А	EL	6.9	0.80	0.273	1.22	А	EL	34.5
		SNS5A	35.550		1.191	42.346	1.4	0.273	1.53	А	EL	34.5	0.507	2.06	А	EL	6.9	0.80	0.273	1.19	А	EL	34.5
		SNS6A	39.950		1.095	43.747	1.4	0.273	1.41	А	EL	34.5	0.507	1.88	А	EL	6.9	0.80	0.273	1.10	А	EL	34.5
LEGAL		SNS7B	42.000		1.043	43.801	1.4	0.273	1.34	А	EL	34.5	0.507	1.85	А	EL	6.9	0.80	0.273	1.04	А	EL	34.5
LOAD		TNAGRIT3	33.000		1.336	44.087	1.4	0.273	1.72	А	EL	34.5	0.507	2.23	А	EL	6.9	0.80	0.273	1.34	А	EL	34.5
RATING		TNT4A	33.075		1.342	44.401	1.4	0.273	1.72	А	EL	34.5	0.507	2.17	А	EL	6.9	0.80	0.273	1.34	А	EL	34.5
		TNT6A	41.600		1.1	45.746	1.4	0.273	1.41	А	EL	34.5	0.507	1.98	А	EL	6.9	0.80	0.273	1.10	А	EL	34.5
	ST	TNT7A	42.000		1.106	46.462	1.4	0.273	1.42	А	EL	34.5	0.507	1.94	А	EL	6.9	0.80	0.273	1.11	А	EL	34.5
		TNT7B	42.000		1.147	48.18	1.4	0.273	1.47	А	EL	34.5	0.507	1.8	А	EL	6.9	0.80	0.273	1.15	А	EL	34.5
		TNAGRIT4	43.000		1.089	46.838	1.4	0.273	1.4	А	EL	34.5	0.507	1.74	А	EL	6.9	0.80	0.273	1.09	А	EL	34.5
		TNAGT5A	45.000		1.026	46.175	1.4	0.273	1.32	А	EL	34.5	0.507	1.74	А	EL	6.9	0.80	0.273	1.03	А	EL	34.5
		TNAGT5B	45.000	3	1.013	45.579	1.4	0.273	1.3	А	EL	34.5	0.507	1.66	А	EL	6.9	0.80	0.273	1.01	А	EL	34.5



	DRAWN BY :	COY M. PATTON	DA [·]	TE : <u>12/2018</u>	
	CHECKED BY :	VINCENT M.WU		TE : <u>1/2019</u>	_
	DESIGN ENGINEER (OF RECORD: VINCENT	M.WU DA	TE : <u>1/2019</u>	_
Ρ	lotted By: CPatton	4/22/2	019 10 : 23 : 1	.8 AM 1	R:\17BP\10R1

<u>IARY</u>



DRMP JOB NUMBER: 15-0116.008

LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

NUMBER

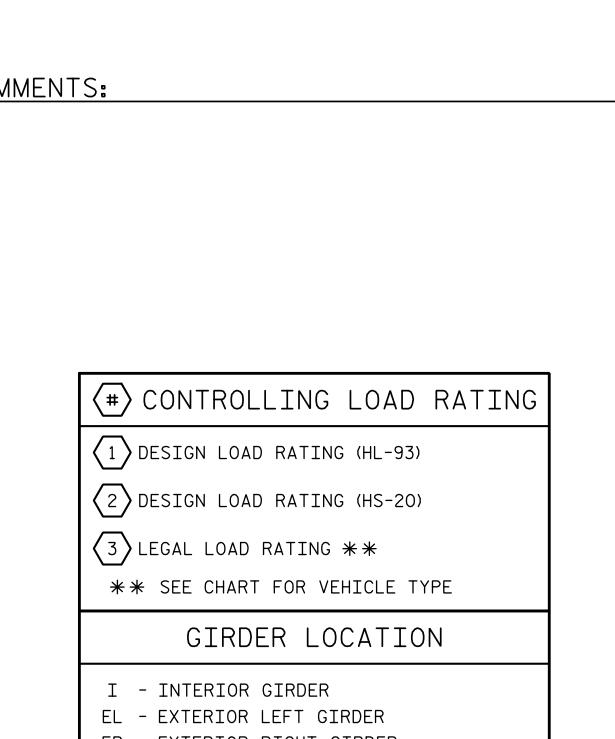
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MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

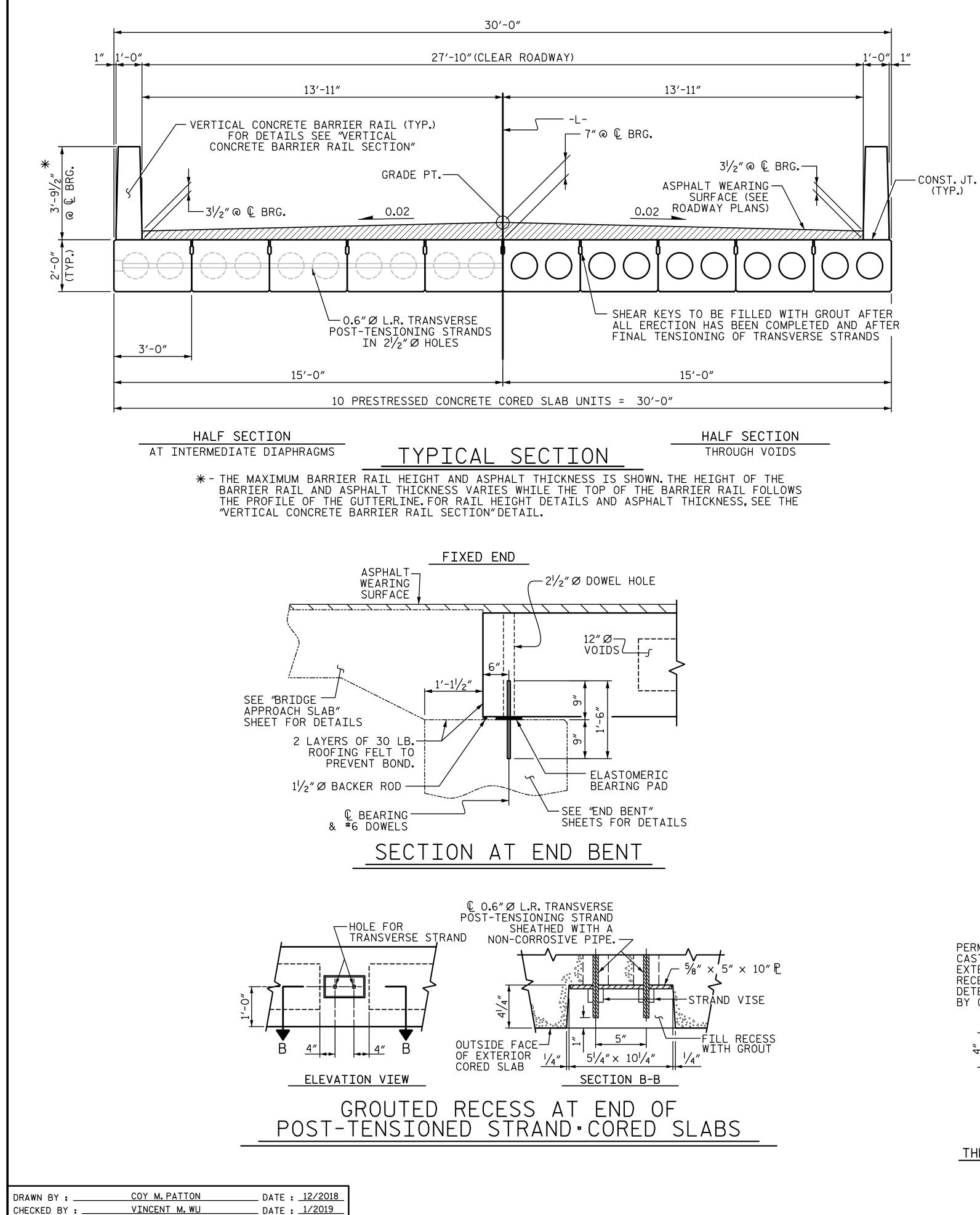
COMMENTS:

- 2.





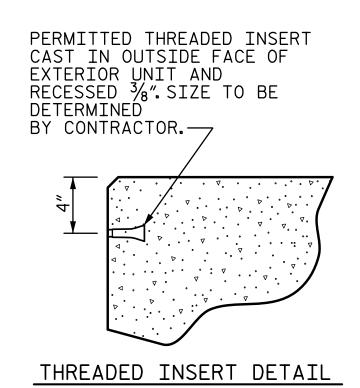
PROJECT NO. 178P.10.R.107 UNION _ COUNTY STATION: 16+76.13 -L-STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD LRFR SUMMARY FOR 70'-O"CORED SLAB UNIT 90° SKEW (NON-INTERSTATE TRAFFIC) Docusigned by: SEAL 4/22/2019 VINCINE M. WO.42684 REVISIONS SHEET NO. S-03 NO. BY: DATE: BY: DATE: TOTAL SHEETS 14



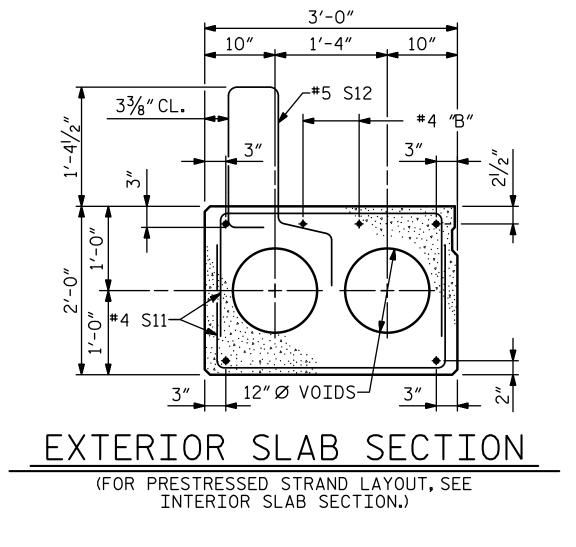
4/22/2019 10**:**23**:**19 AM

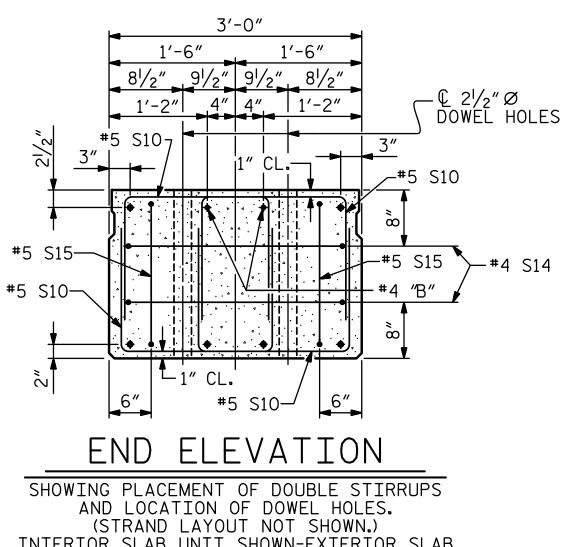
DESIGN ENGINEER OF RECORD: _____VINCENT M. WU DATE : 1/2019

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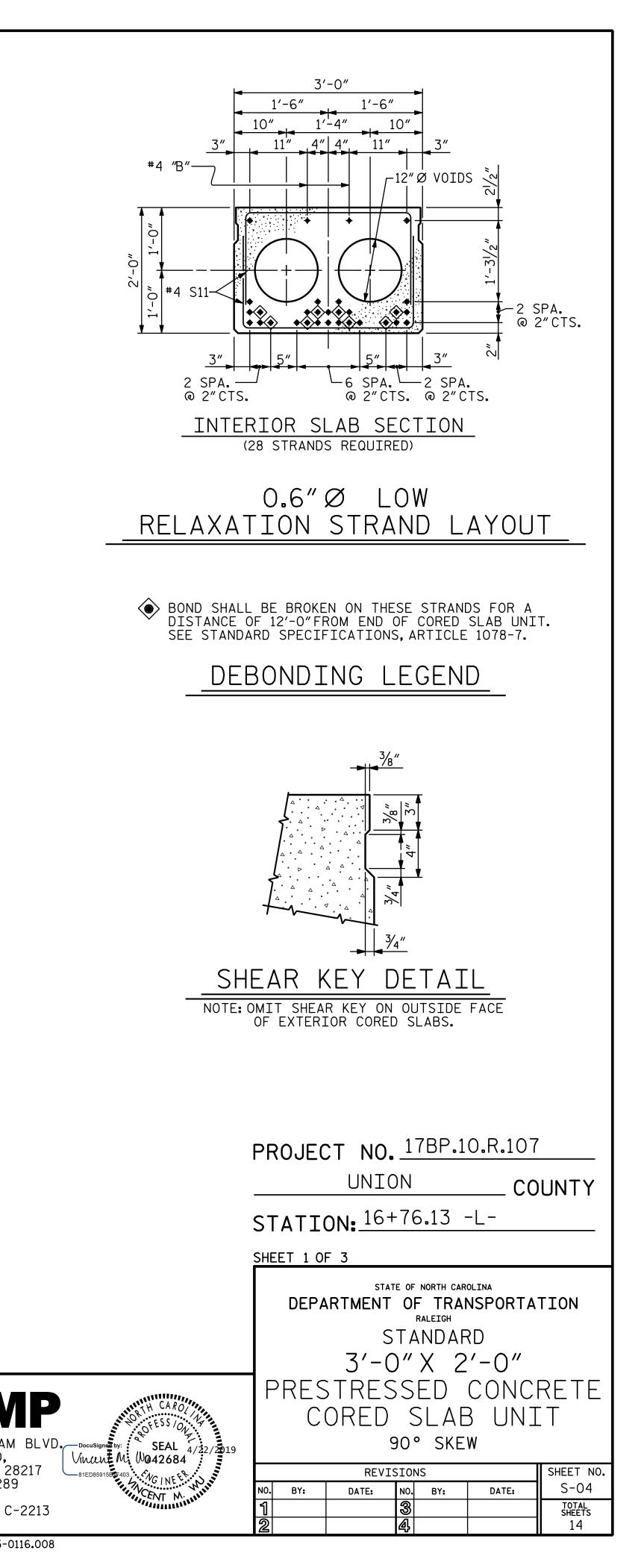


4235 SOUTH STREAM BLVD. SUITE 150, CHARLOTTE, NC 28217 (704) 332-2289 NC LICENSE NO. C-2213

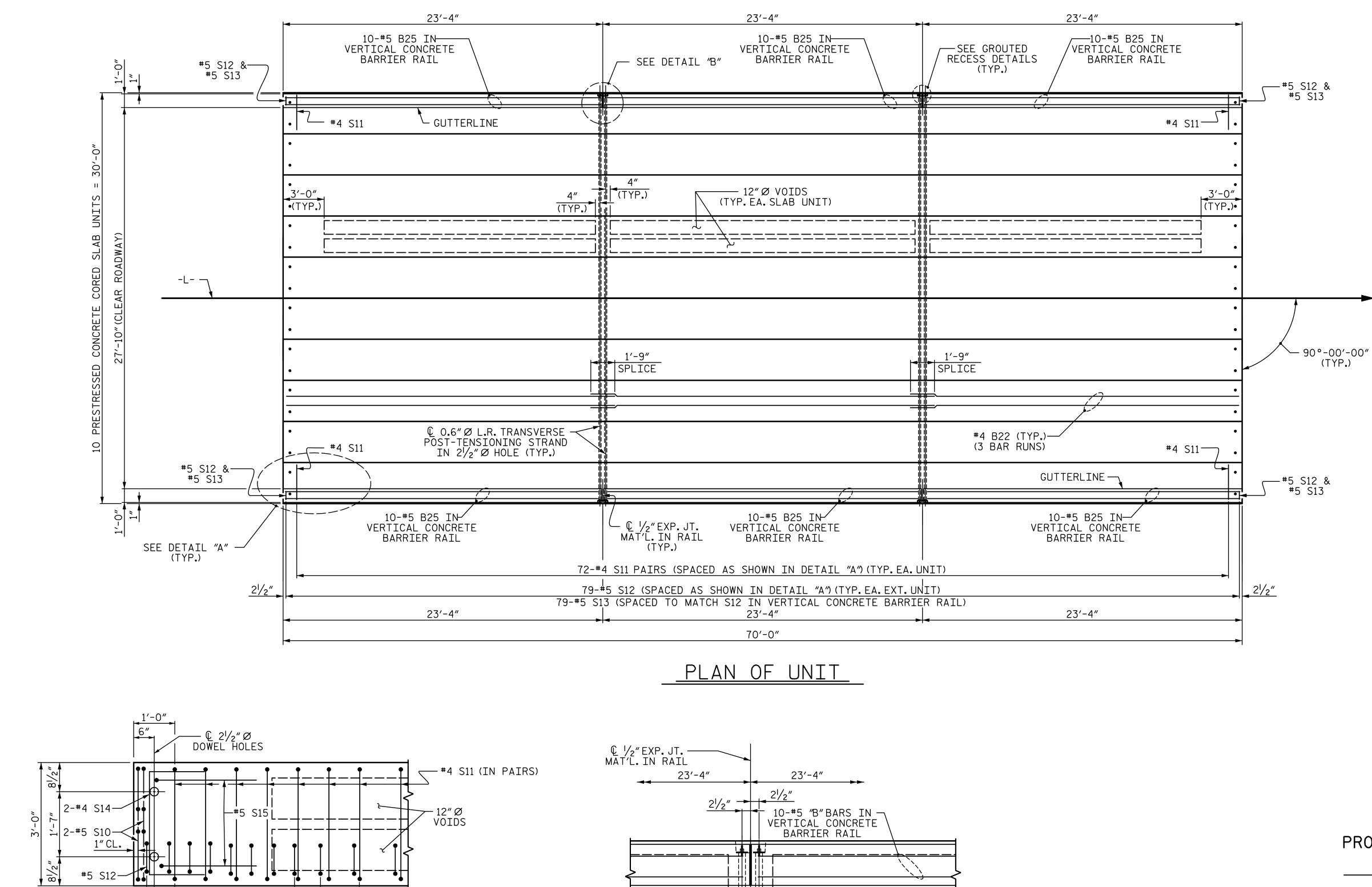




INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.



•



DRAWN BY : COY M.P.	ATTON	DATE :12/20	018_
CHECKED BY : VINCENT	M. WU	DATE :1/20	9
DESIGN ENGINEER OF RECORD:	VINCENT M.WU	DATE : <u>1/20</u>	.9
lotted By: CPatton	4/22/2019 10	0:23:20 AM	R:\17BP\10R107\Structures\100

 $2^{1}/2^{\prime\prime}$

7-#4 S11 PAIRS

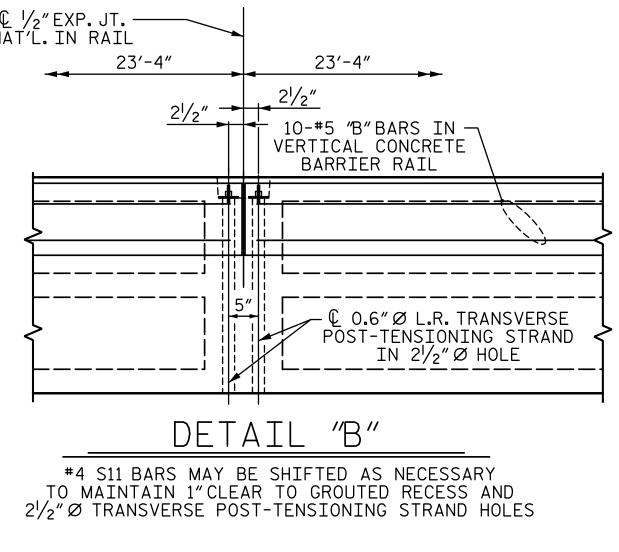
@ 9″CTS.

DETAIL "A"

(TYPICAL EACH END OF UNIT) NOTE: EXTERIOR UNIT SHOWN - INTERIOR UNIT SIMILAR EXCEPT OMIT #5 S12 BARS.

8-#5 S12 @ 6″CTS.

31/2"



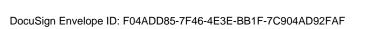


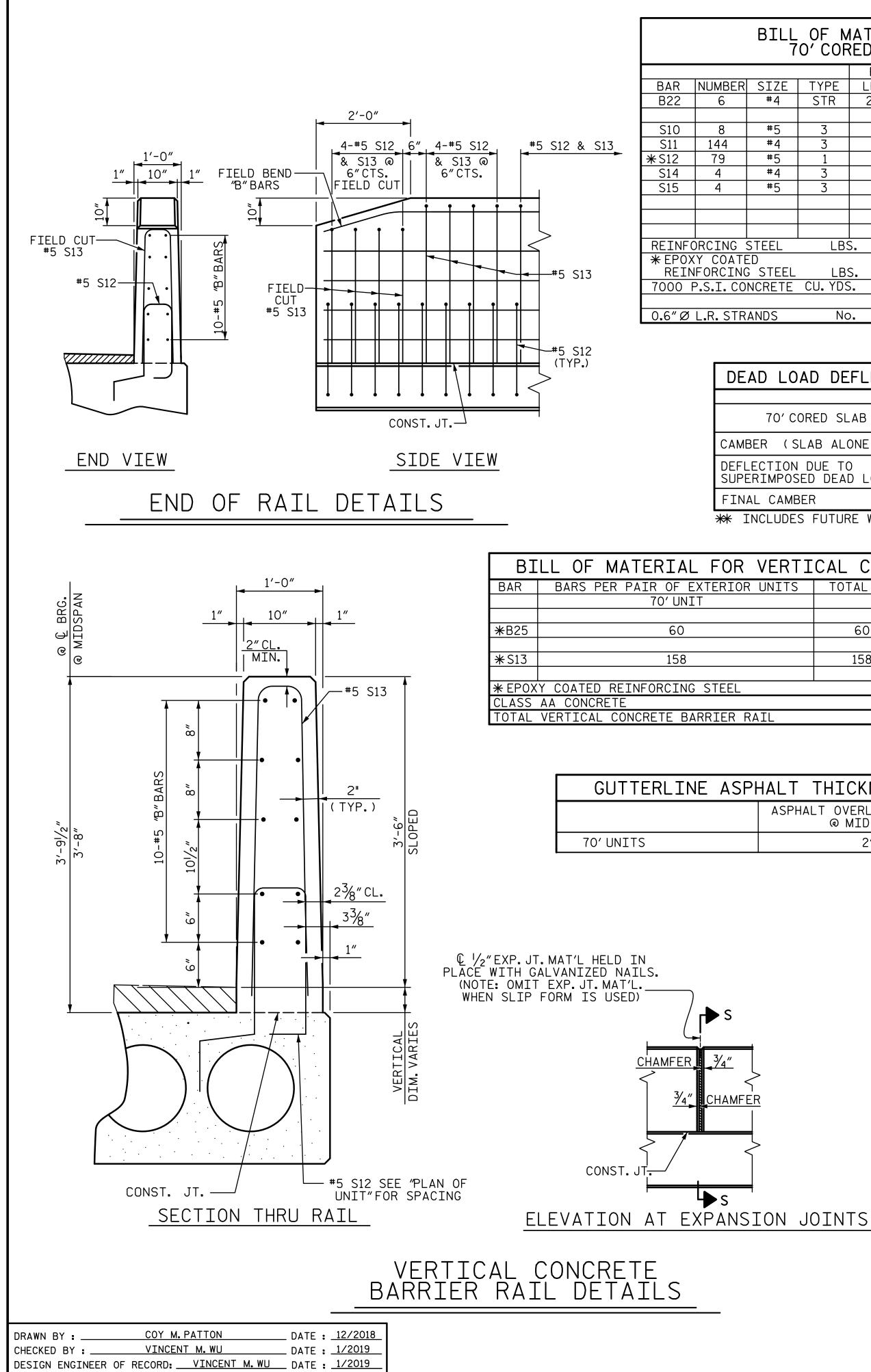
#4 S11 PAIRS

@ 1'-0"CTS.

#5 S12 @ 1'-0"CTS. _

		UNI		CC	, UNTY
	STATI	[ON: 16-	+76.13	<u>-L-</u>	
	SHEET 2	OF 3			
	DEF		TE OF NORTH CAR OF TRA RALEIGH		TION
	PL	AN O	F 70'	-0″ UI	VIT
AM BLVD. Docusign by: SEAL 4/22/2019			CLEAR F 90° SKE		,
28217 B1ED85915 2F403		REVI	ISIONS		SHEET NO.
289 THE MONTH	NO. BY:	DATE:	NO. BY:	DATE:	S-05
. C-2213	1 2		3 4		total sheets 14
5-0116 008					





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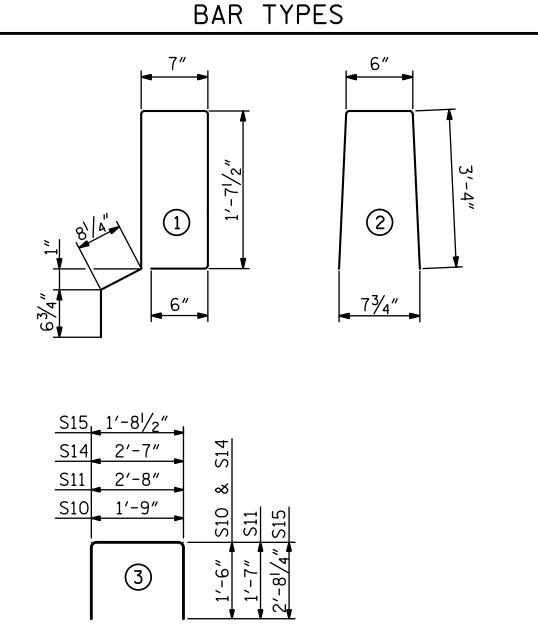
	BILL OF MATERIAL FOR ONE 70' CORED SLAB UNIT								
			EXTERI	OR UNIT	INTERI	OR UNIT			
BER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT			
	#4	STR	24'-6"	98	24'-6″	98			
	#5	3	4'-9"	40	4'-9"	40			
	#4	3	5'-10"	561	5'-10″	561			
	#5	1	5′-7″	460					
	#4	3	5′-7″	15	5′-7″	15			
	#5	3	7'-1″	30	7'-1"	30			
IG :	STEEL	LBS	5.	744		744			
ATE	ED								
	<u>S STEEL</u>			460					
C0	NCRETE	CU. YDS	,) .	11.8		11.8			
STR	ANDS	Nc).	28		28			

DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0" × 2'-0"
70' CORED SLAB UNIT	0.6″ØL.R. STRAND
AMBER (SLAB ALONE IN PLACE)	2¹⁄₄″ ♦
EFLECTION DUE TO UPERIMPOSED DEAD LOAD	3∕4″ ↓
INAL CAMBER	1 ∕2″ ♦

** INCLUDES FUTURE WEARING SURFACE

DR VERTICAL CONCRETE BARRIER RAIL							
IOR UNITS	INITS TOTAL NO. SIZE TYPE LENGTH WEIGH						
	1434						
	#5	2	7'-2″	1181			
EL	EL LBS. 2615						
	CU.YDS. 18.1						
IR RAIL	R RAIL LN.FT. 140						

S PF	ALT THICKNESS & RAI	L HEIGHT
	ASPHALT OVERLAY THICKNESS @ MID-SPAN	RAIL HEIGHT @ MID-SPAN
	2″	3'-8"

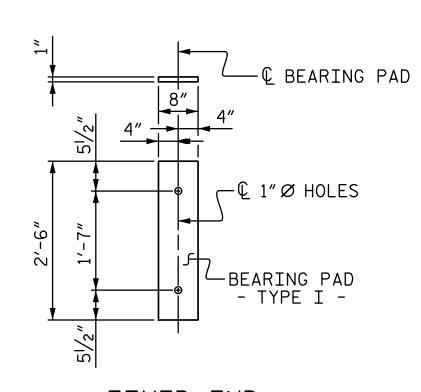


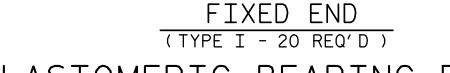
ALL BAR DIMENSIONS ARE OUT TO OUT

CONCRETE RELEA	ASE STRENGTH
UNIT	PSI
70' UNITS	5500

GRADE 270 STRANDS					
0.6″ØL.R.					
AREA (SQUARE INCHES)	0.217				
ULTIMATE STRENGTH (LBS.PER STRAND)	58,600				
APPLIED PRESTRESS (LBS.PER STRAND)	43,950				

CORED	CORED SLABS REQUIRED					
NUMBER LENGTH TOTAL LENGTH						
70' UNIT						
EXTERIOR C.S.	2	70'-0″	140'-0"			
INTERIOR C.S.	8	70'-0″	560'-0"			
TOTAL	10		700′-0″			





ELASTOMERIC BEARING DETAI

ELASTOMER IN ALL BEARINGS SHALL BE 60 DUROMETER HAR



21/2" $2^{1/2''}$ 2″ | 21/2'

SECTION S-S AT DAM IN OPEN JOINT (THIS IS TO BE USED ONLY WHEN SLIP FORM IS USED)

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE 21/2" Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS.AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMI TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

MAINTAIN A SYMMETRIC TENSION FORCE BETWEEN EACH PAIR OF TRANSVERSE POST TENSIONING STRANDS IN THE DIAPHRAGM.

THE #4 S11 STIRRUPS MAY BE SHIFTED AS NECESSARY TO MAINTAIN 1" CLEAR TO THE GROUTED RECESS.

FOR GROUT FOR STRUCTURES. SEE SPECIAL PROVISIONS.

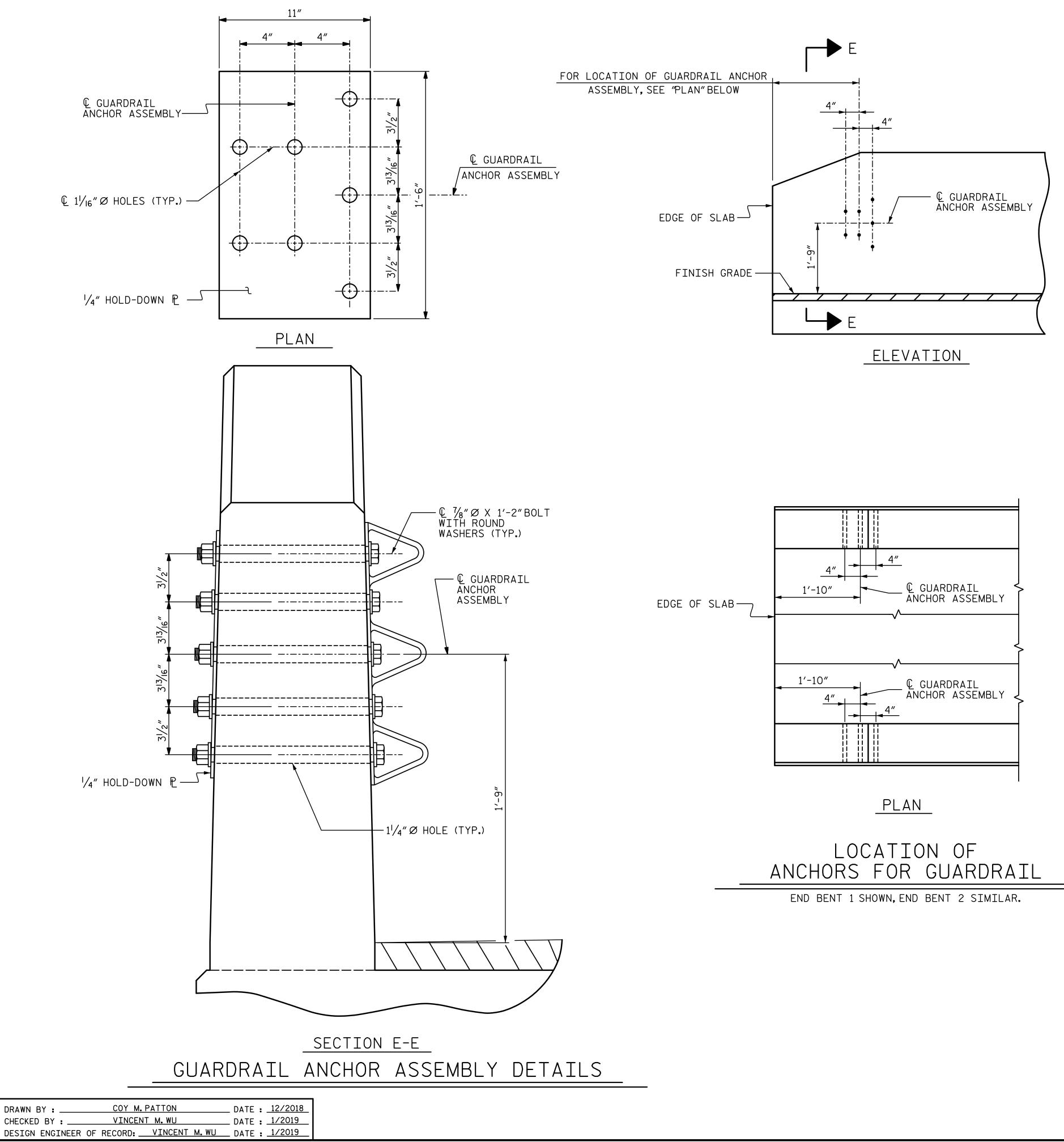
THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-O"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

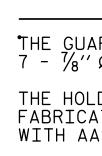
THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

	PROJECT NO. 17BP.10.R.107 UNION COUNTY					
	STATION: 16+76.13 -L-					
	SHEET 3 OF 3					
M BLVD. M B	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH 3'-0" X 2'-0" PRESTRESSED CONCRETE CORED SLAB UNIT 90° SKEW					
28217	REVISIONS SHEET NO. NO. BY: DATE: NO. BY: DATE: S-06					
C-2213	NO. BY: DATE: NO. BY: DATE: S=06 1 3 3 TOTAL SHEETS SHEETS 14					
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THE ENGINEER.)

THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT. SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE 1 $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



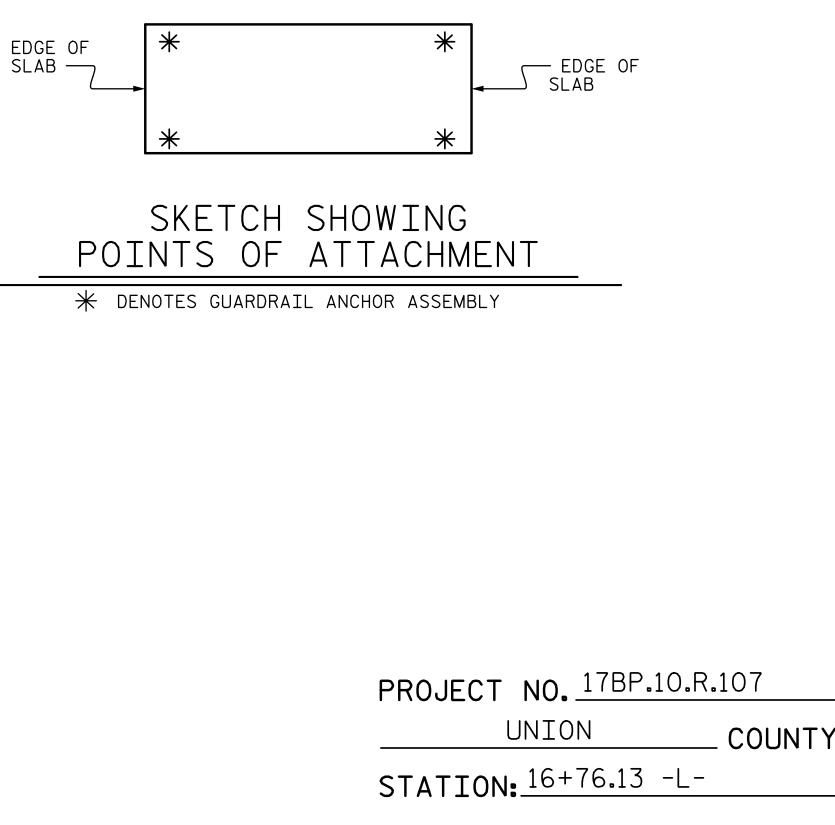
NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 - $\frac{1}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.

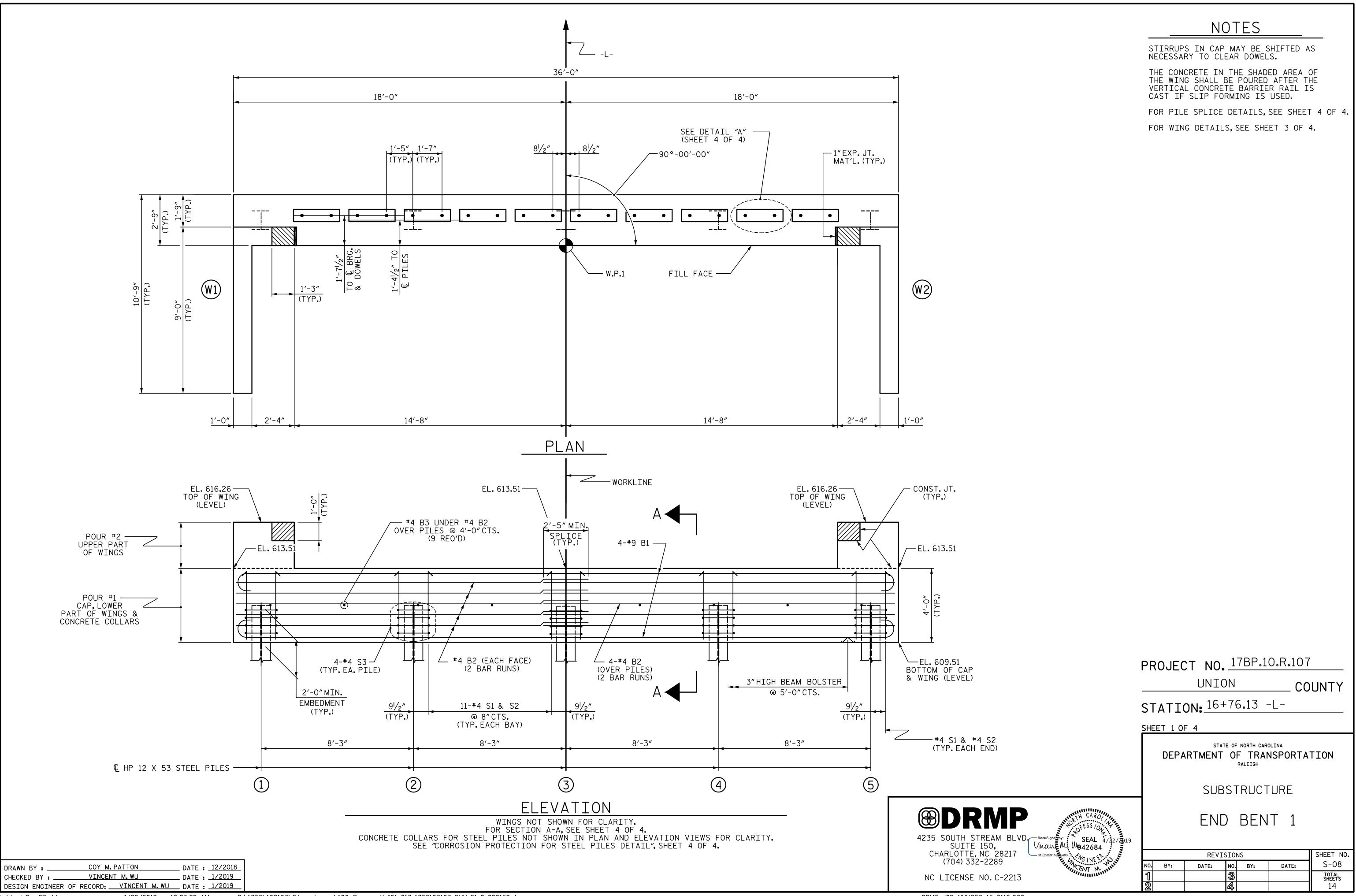
THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE $\frac{7}{8}$ " Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY

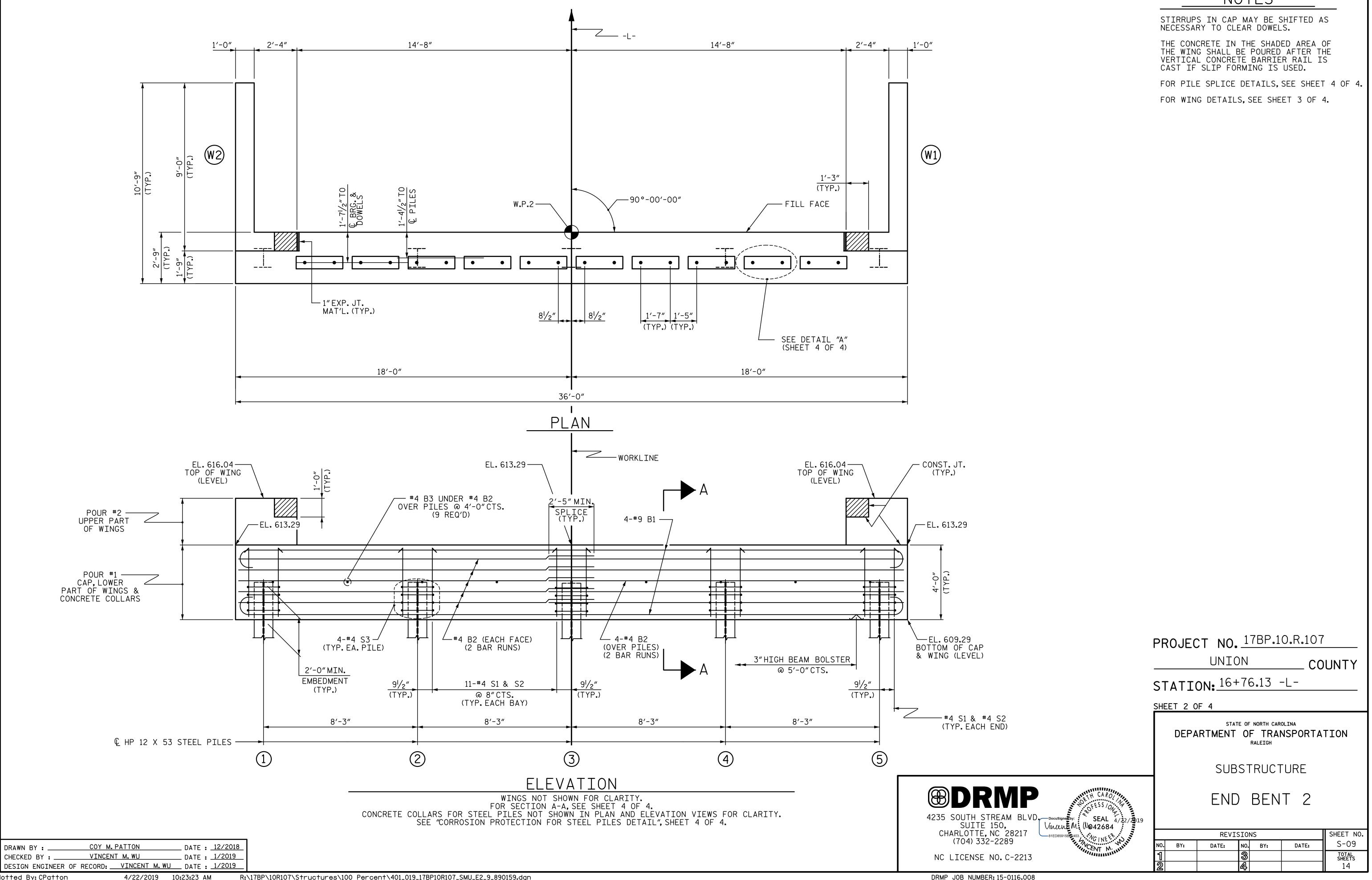
THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD GUARDRAIL ANCHORAGE DETAILS FOR VERTICAL CONCRETE BARRIER RAIL SEAL 4/2 2019 . W042684 Vina REVISIONS SHEET NO. NO. BY: S-07 DATE: DATE: BY: TOTAL SHEETS 14

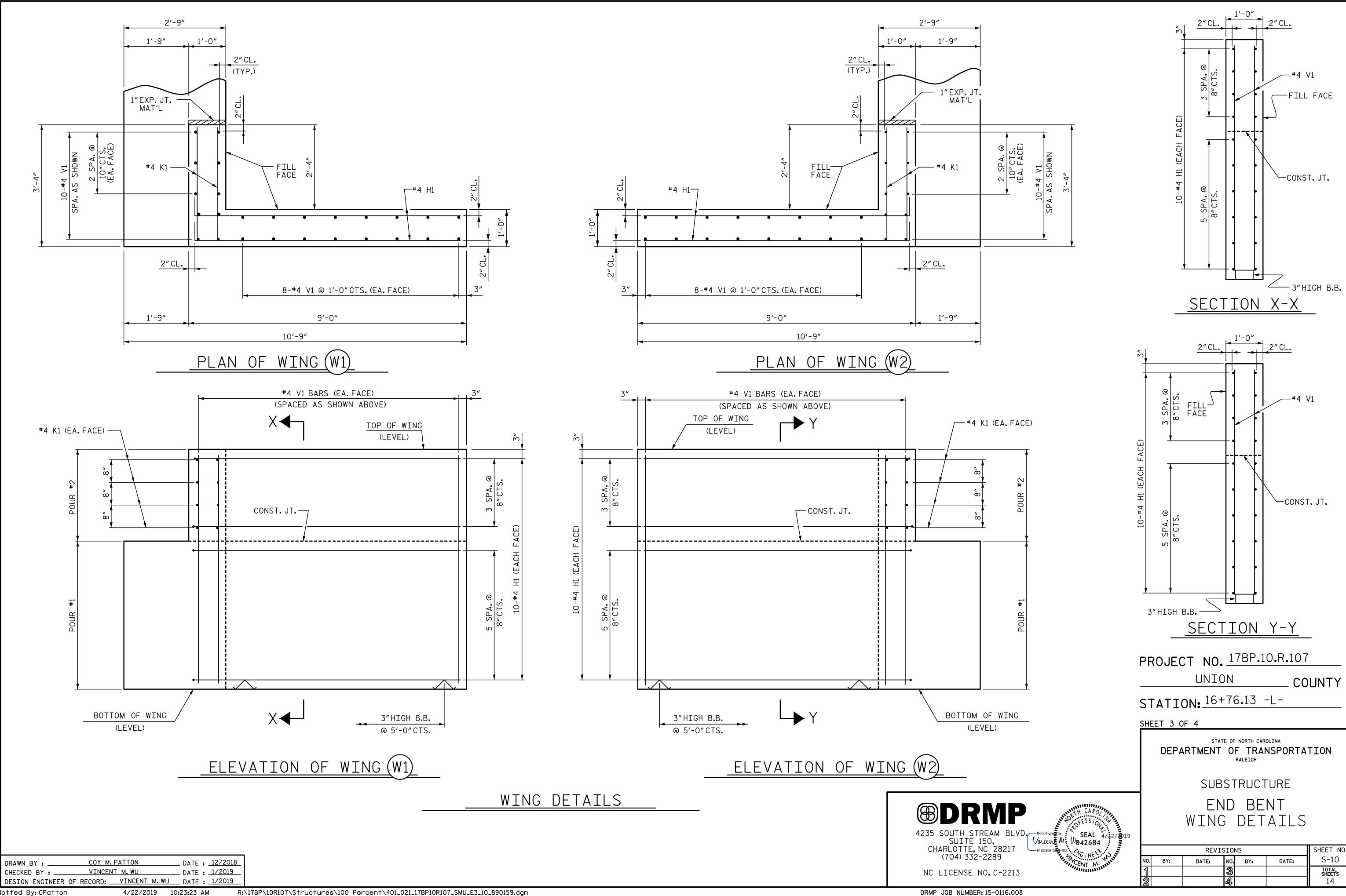


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NOTES



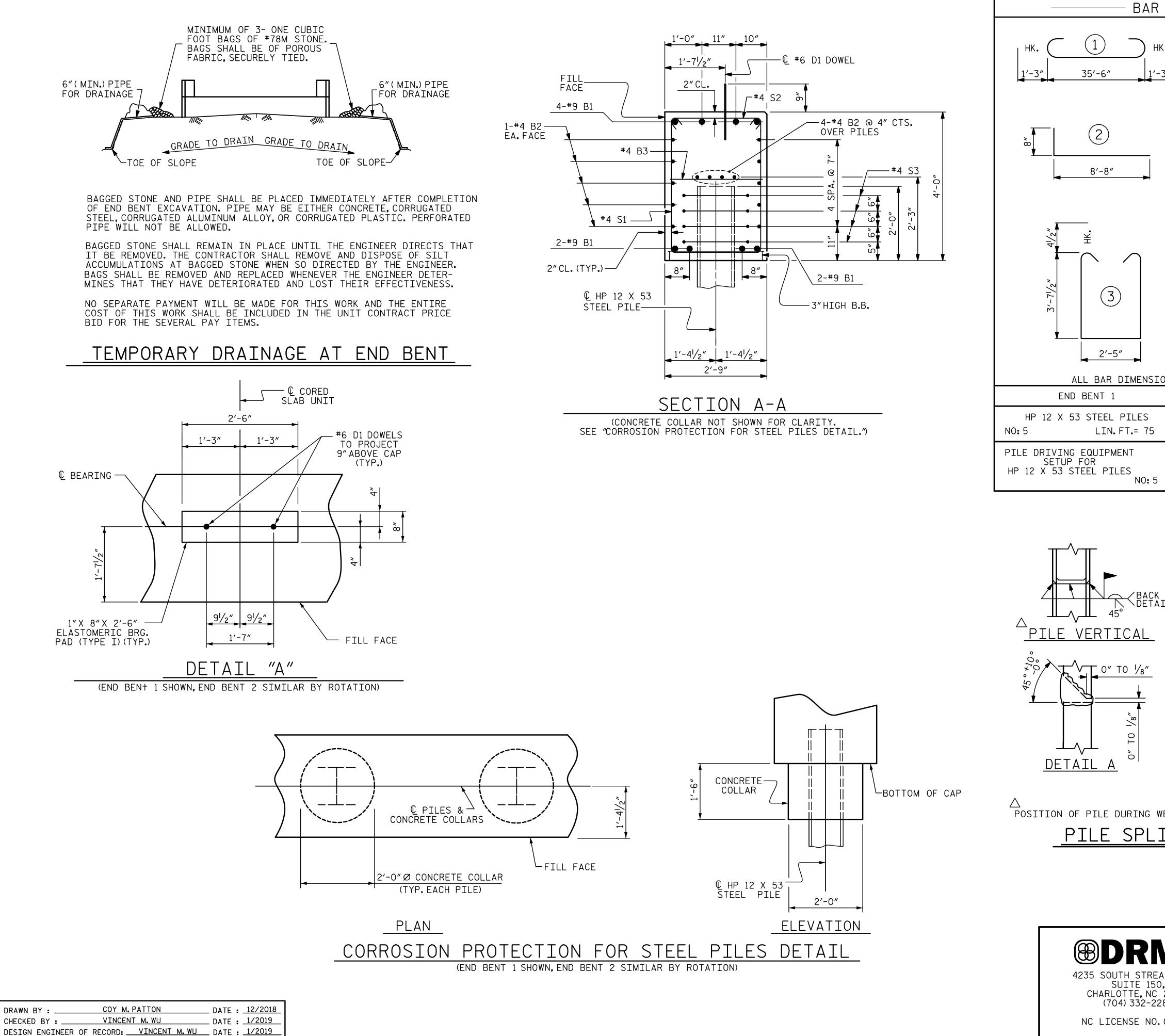
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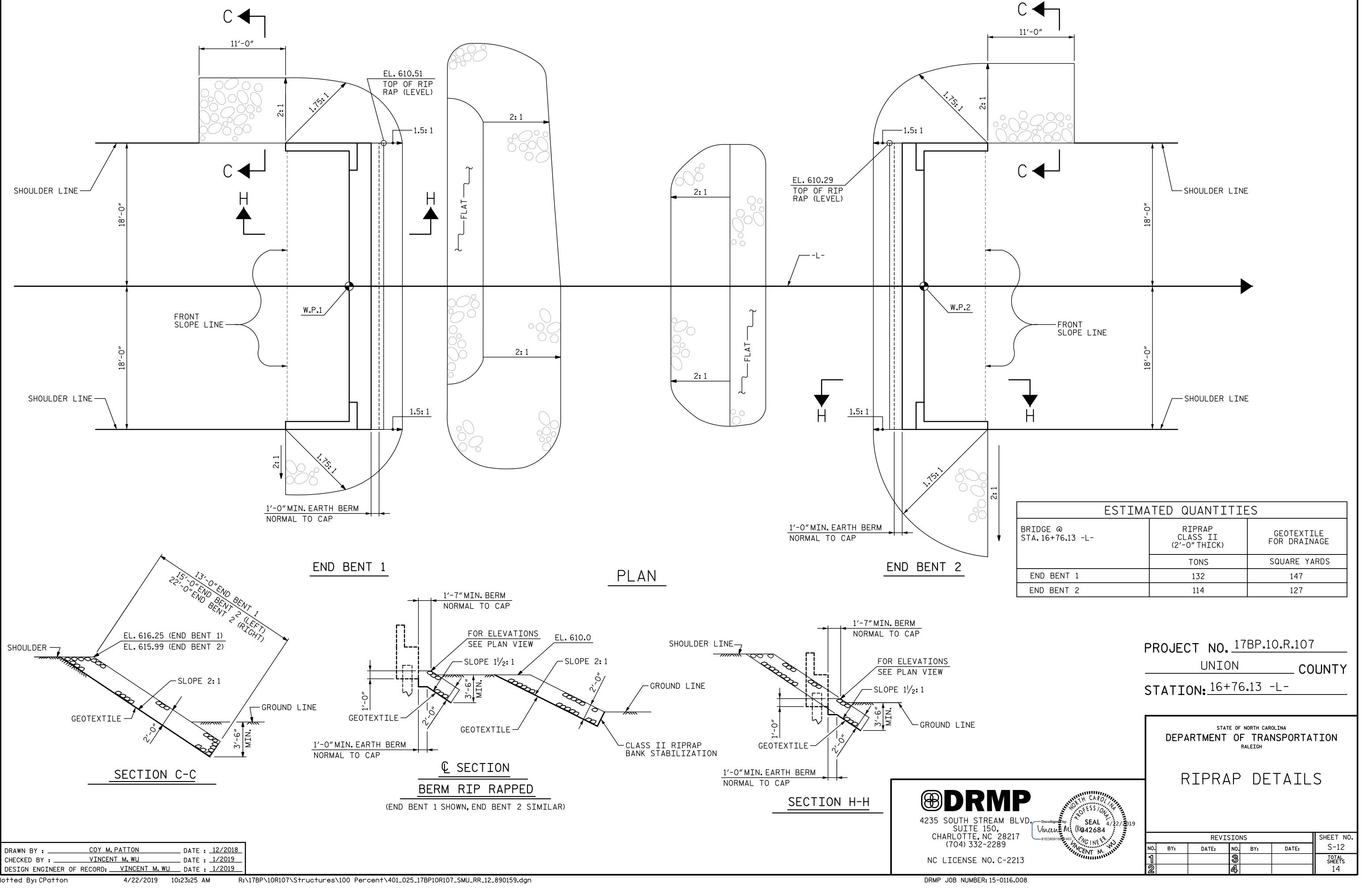
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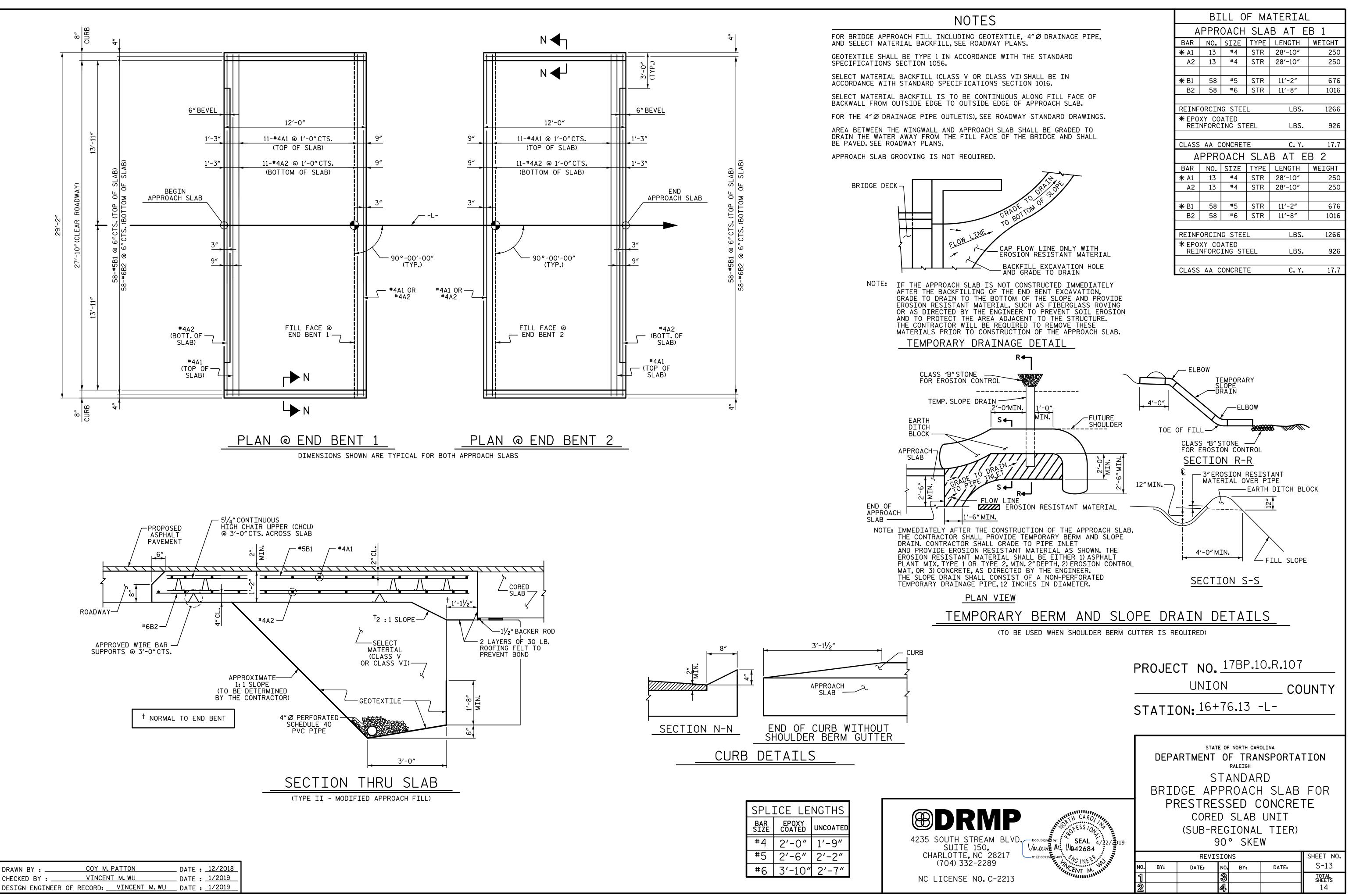
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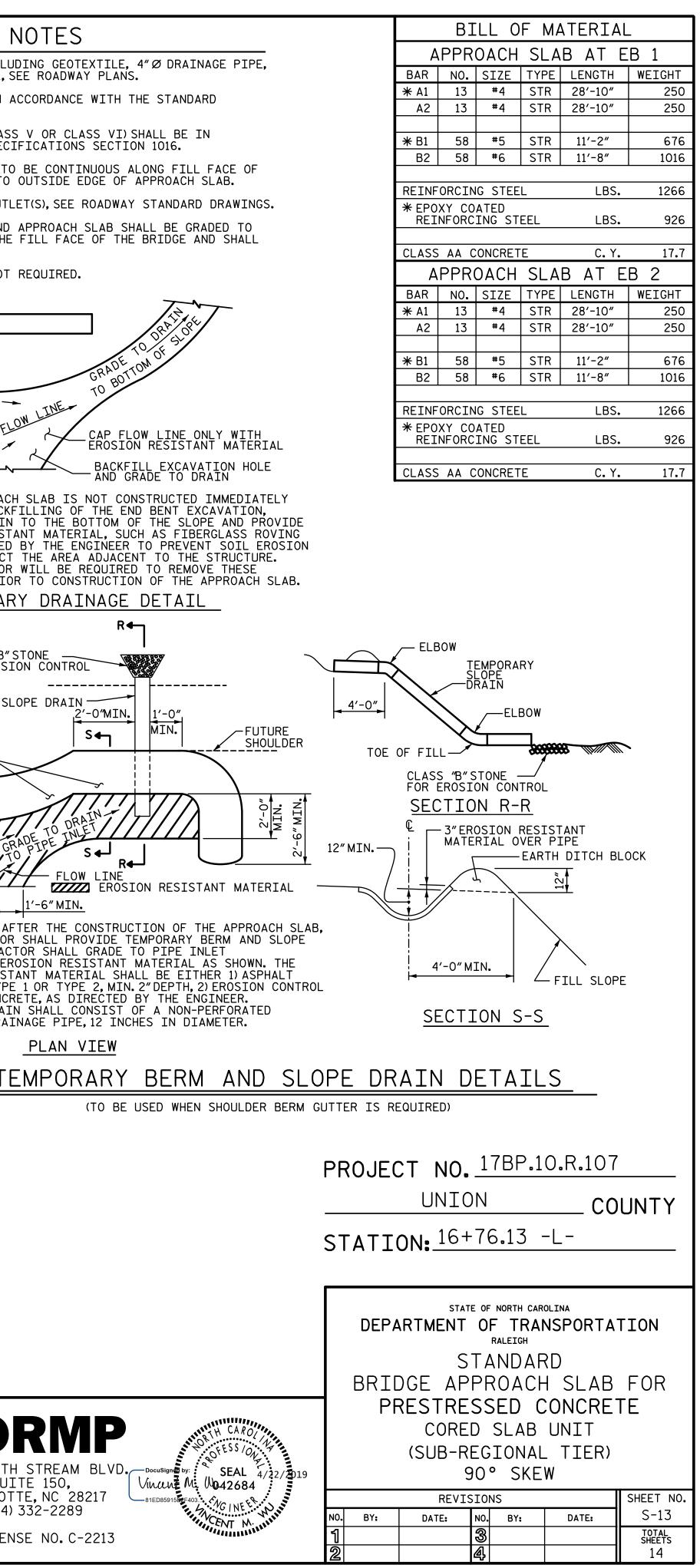
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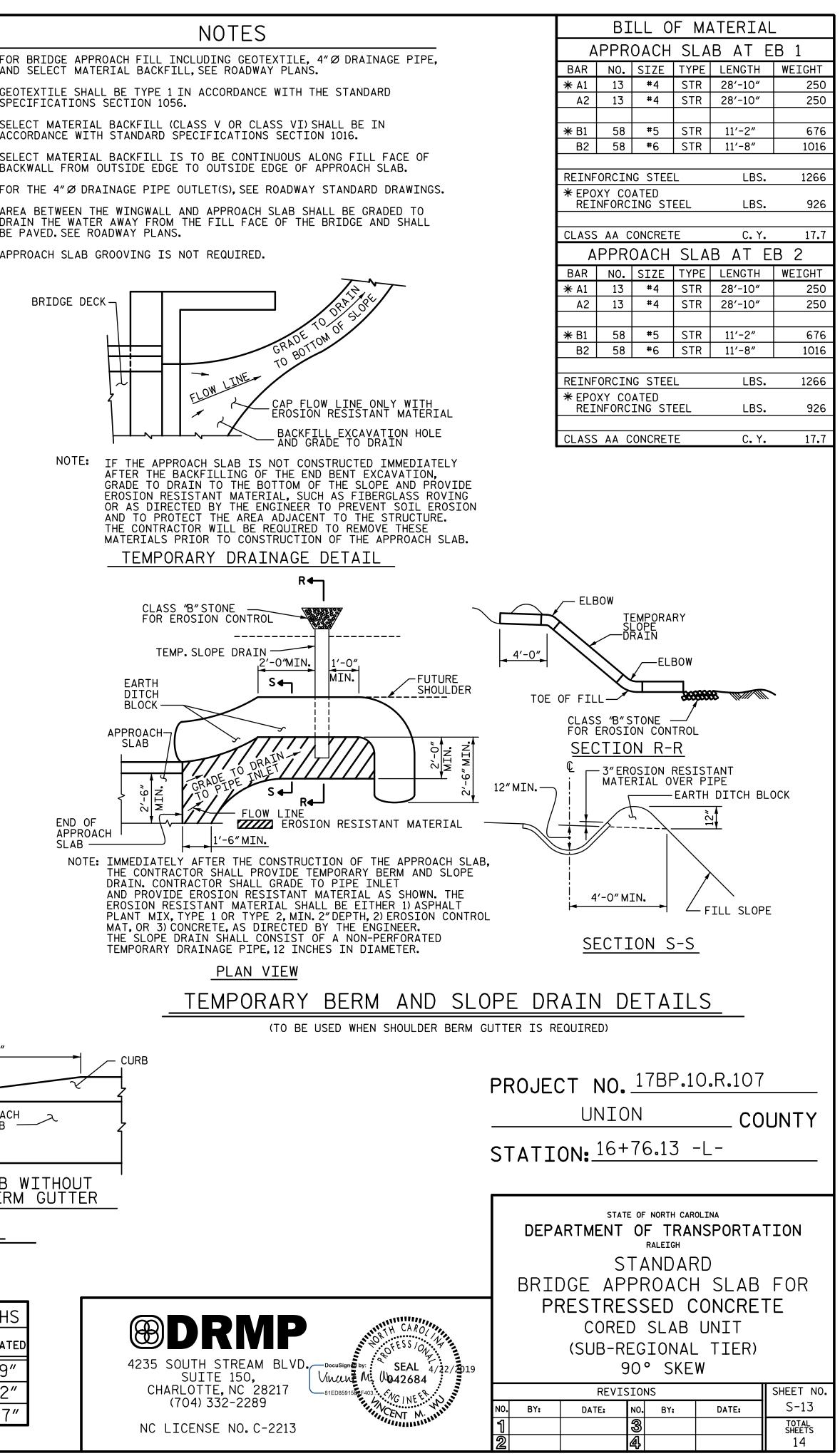
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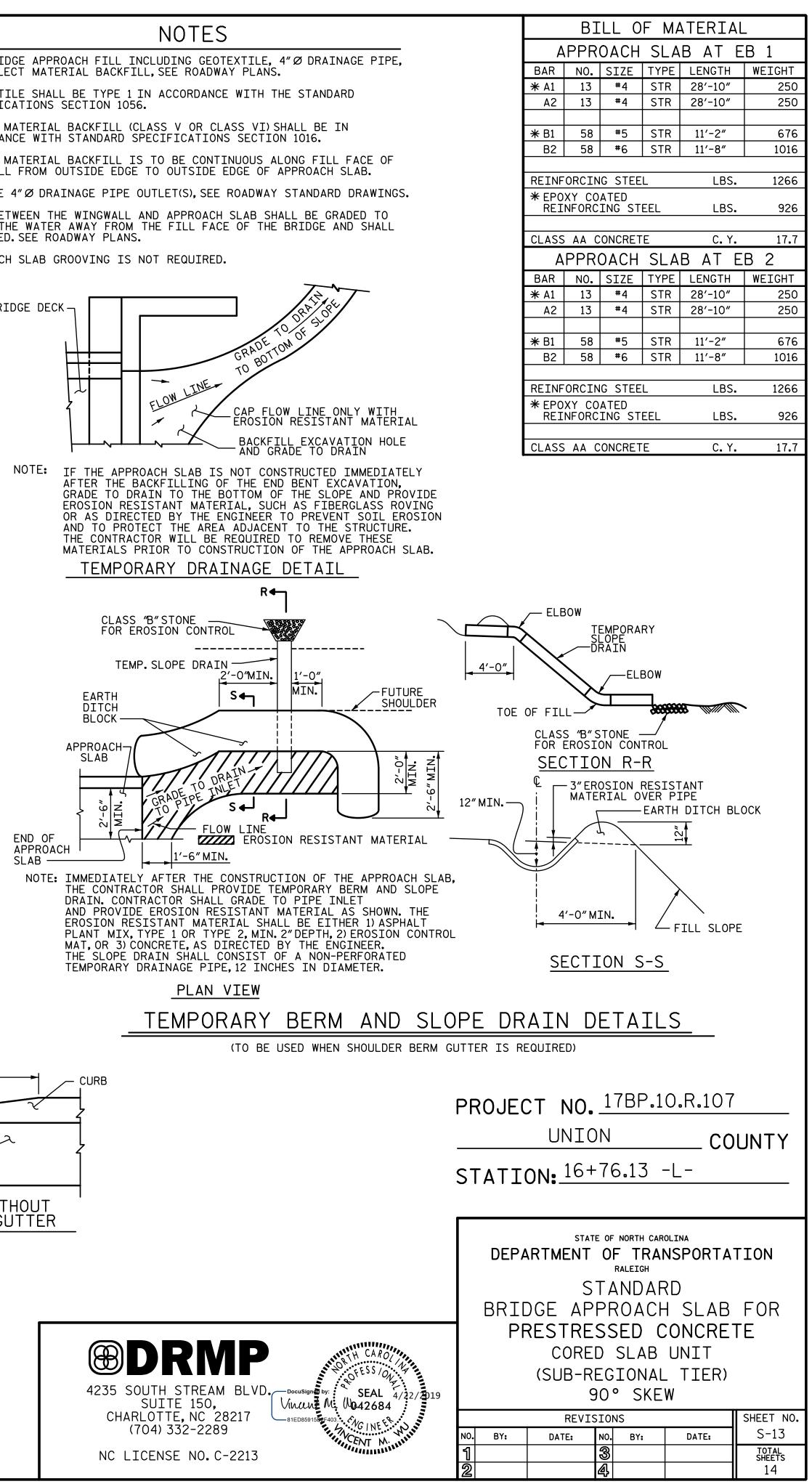


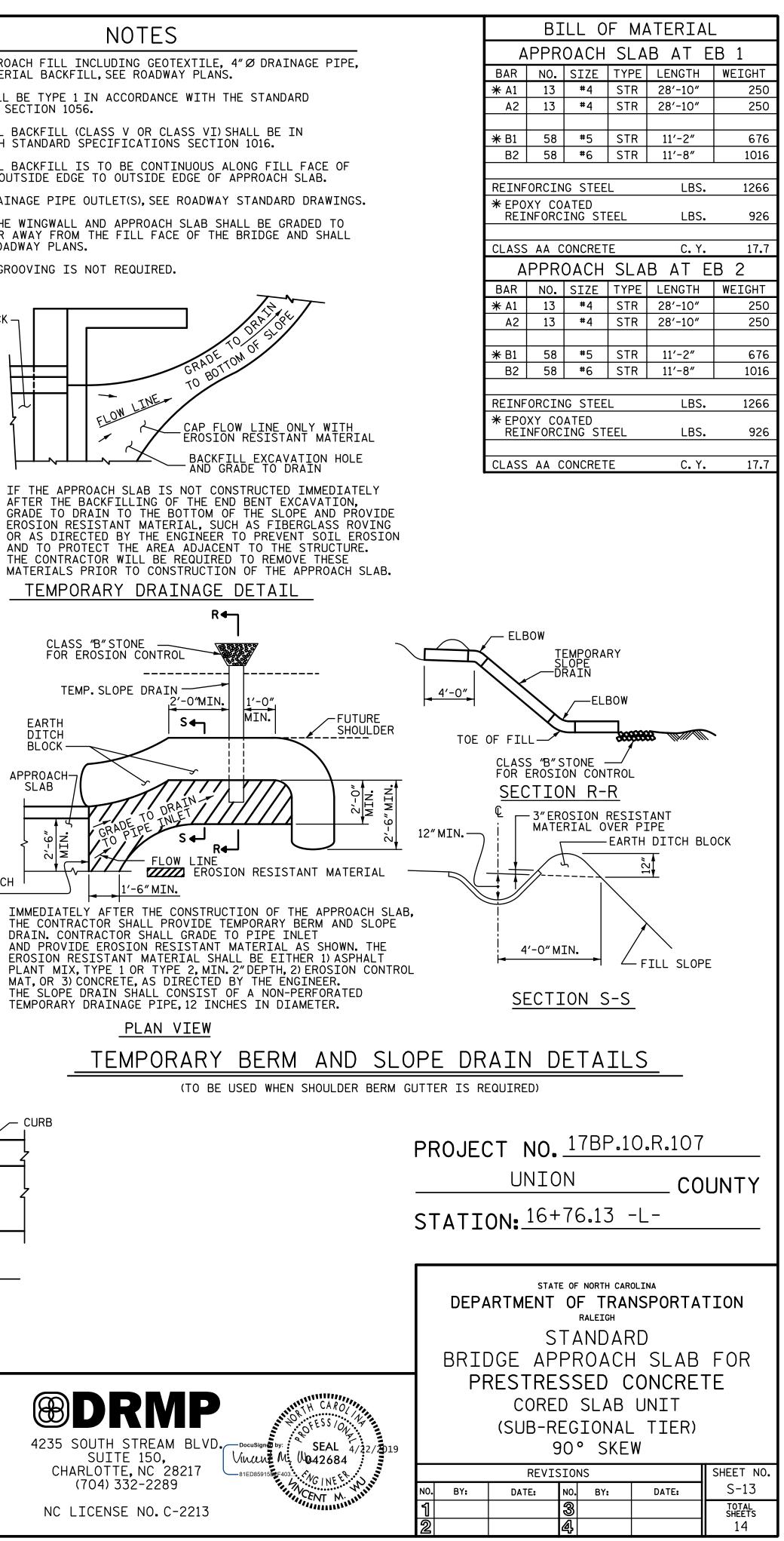


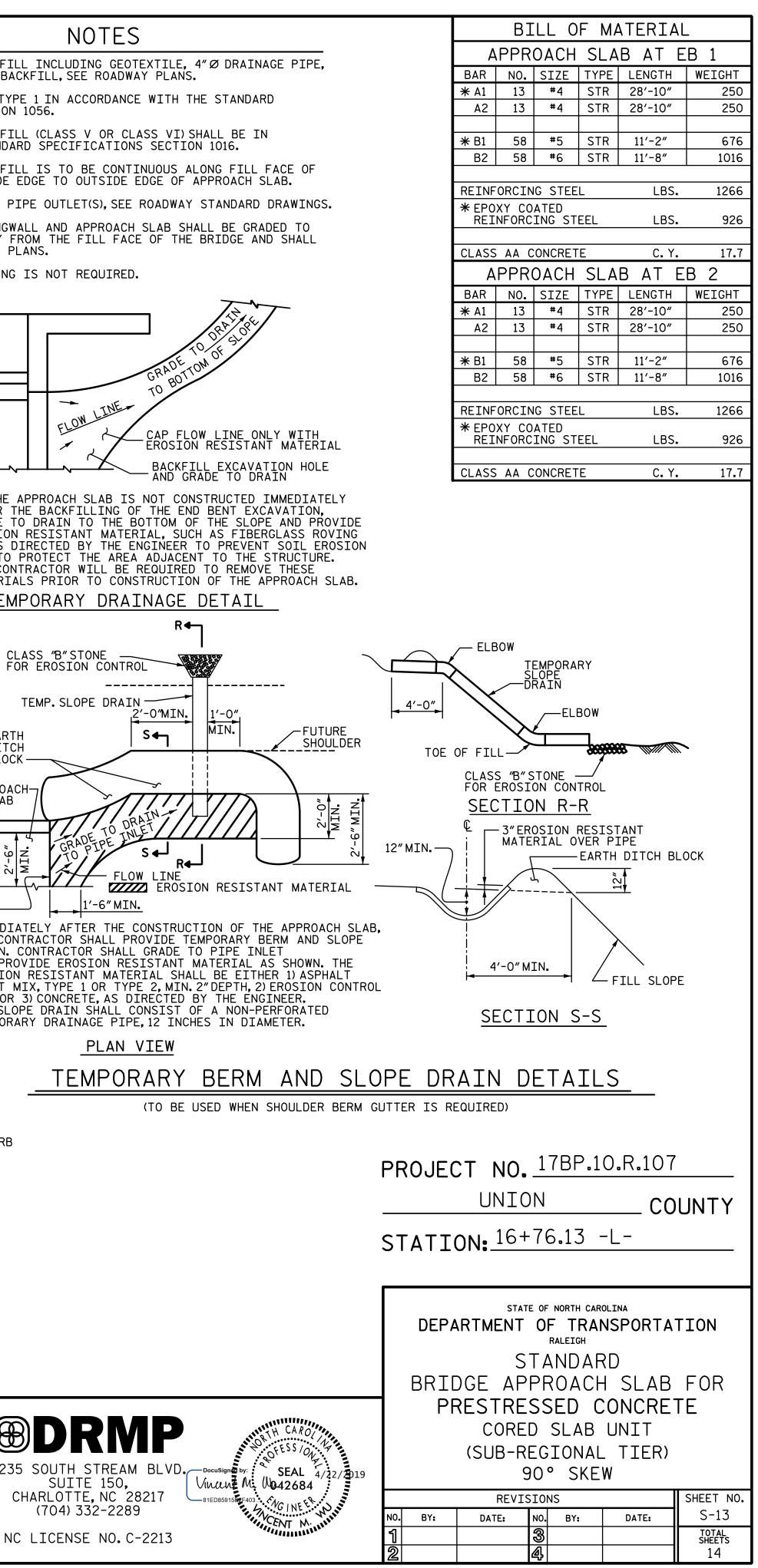
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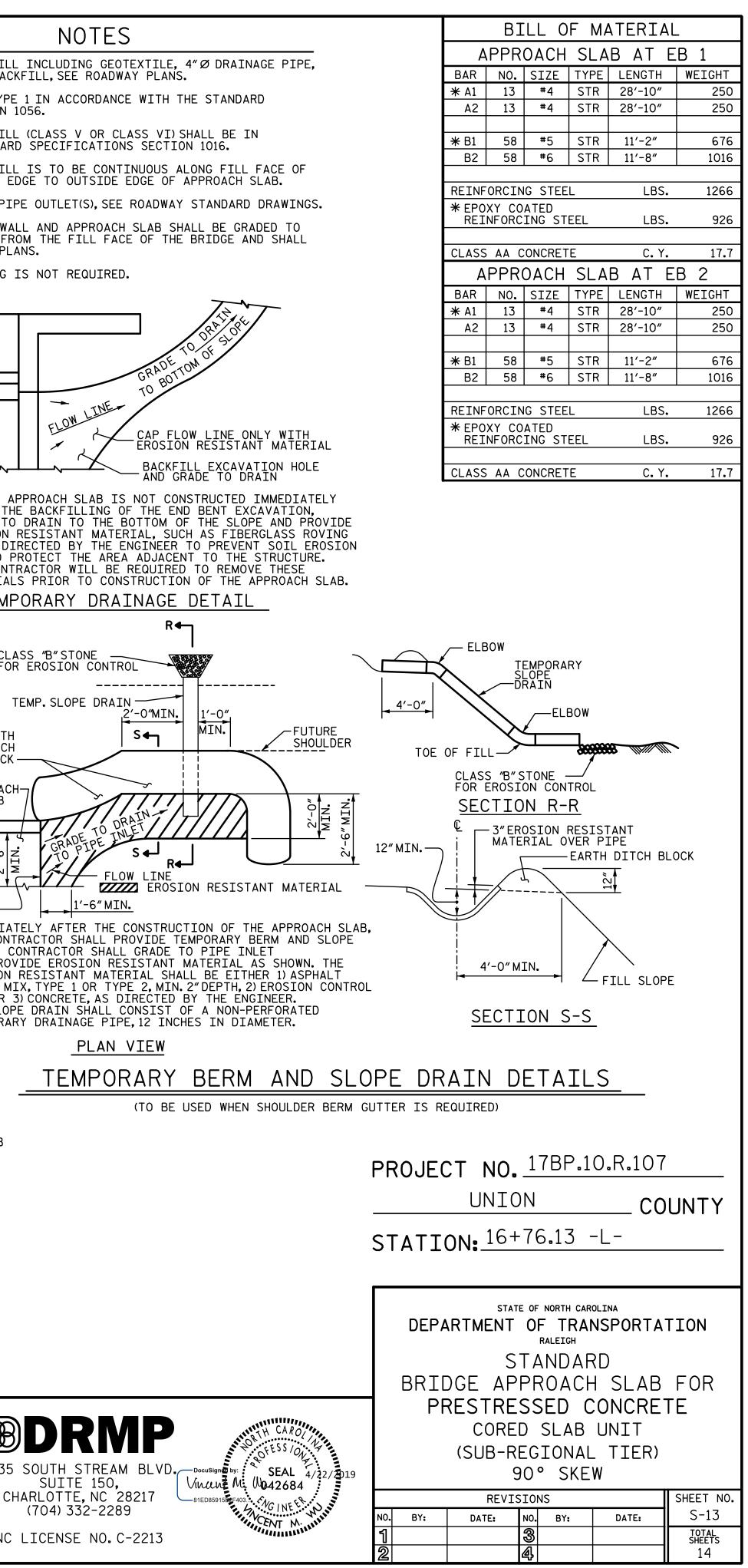


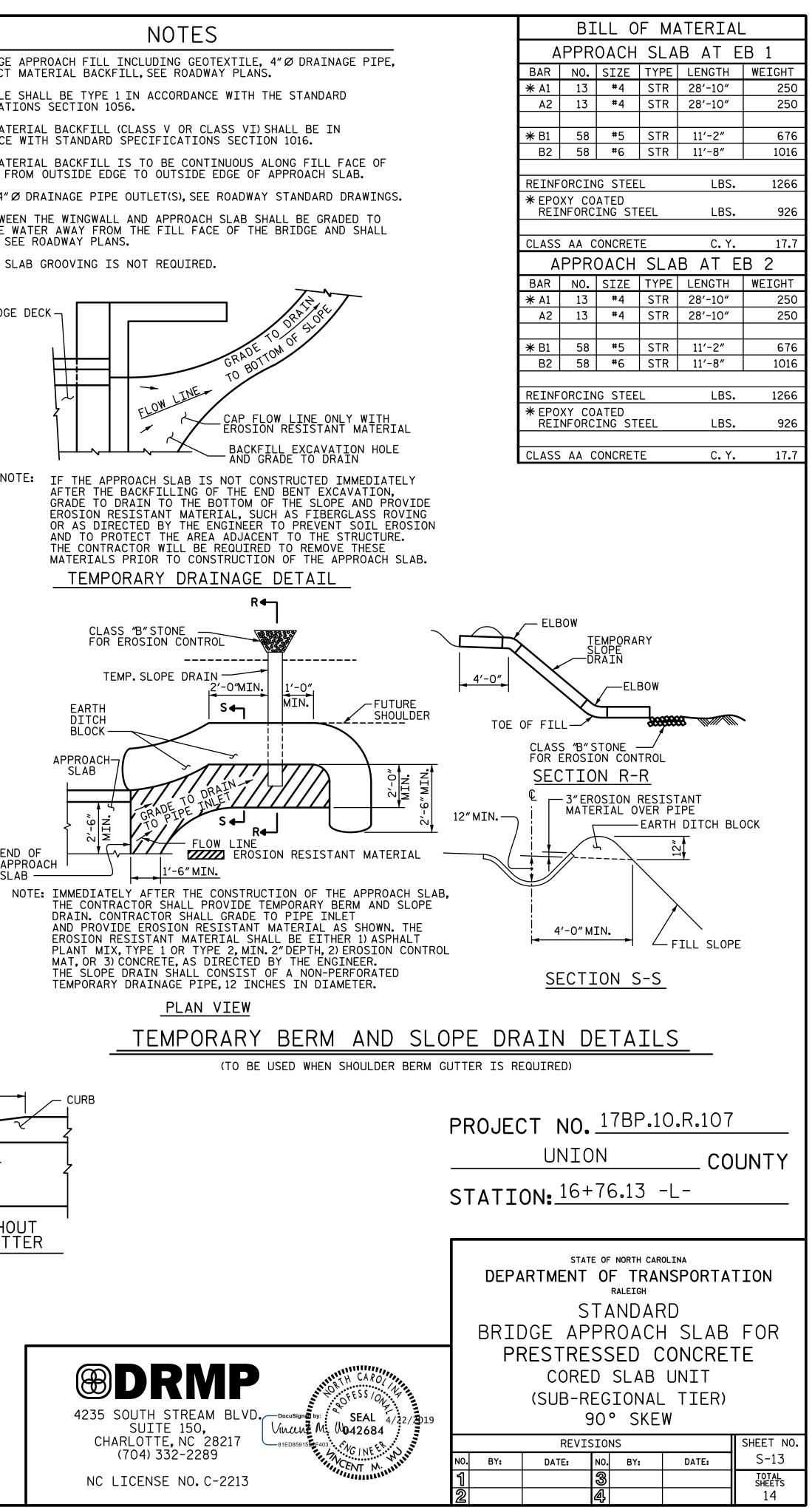












SPLICE LENGTHS					
BAR EPOXY SIZE COATED UNCOATED					
#4	2'-0"	1'-9″			
#5	2'-6″	2'-2″			
#6	3′-10″	2'-7"			

DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36 -	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W -	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50 -	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR	
UNTREATED - EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT.
	(MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS: AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A $\frac{1}{4}$ "RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

DRAWN BY :	COY M.P	ATTON	DATE :	12/2018
CHECKED BY :	VINCENT	M. WU	DATE :	1/2019
DESIGN ENGINEER (OF RECORD:	VINCENT M. WU	DATE :	1/2019

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT. ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK. THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 1/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 1/4" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES.ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16" OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING. GALVANIZING. OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB. METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.



PROJECT	NO.	17BP.10.R.107

UNION COUNTY

STATION: 16+76.13 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD NOTES

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